1. GENERAL NOTES

1.1 FBC REFERS TO 2017 FLORIDA BUILDING CODE.
1.2 –
1.3 COMPACT BACK FILL 5'–0" FROM STRUCTURE. MINIMUM ALLOWABLE BEARING CAPACITY SHALL BE 2000 PSF.
1.4 CONTRACTOR TO VERIFY MANUFACTURED TRUSS PLAN PRIOR TO PLACEMENT OF STEMWALL OR MONOLITHIC FOOTING.
1.5 PLUMBER IS TO INFORM SUPERINTENDENT OF ANY VENTING WHICH UTILIZES A MASONRY WALL TO RESOLVE ANY POSSIBLE STRUCTURAL INTEGRITY ISSUES.

2. CONCRETE/MASONRY NOTES

2.1 ALL CONCRETE SHALL BE F'c=3000PSI.
2.2 MASONRY SHALL USE TYPE S MORTAR. F'm=1900PSI.
2.3 REINFORCING STEEL SHALL SATISFY ASTM A615, GD 60. FOOTING MAY USE GD 40 STEEL.
2.4 WHERE INDICATED ON FLOOR PLANS, PROVIDE CONCRETE FILLED CELL WITH REINFORCING STEEL FROM FOOTING TO TIE BEAM HOOKED & TIED BEFORE INSPECTION. IF GROUT LIFT EXCEEDS 4'–0", AN INSPECTION HOLE TO VERIFY GROUTING SHALL BE PROVIDED AT THE BOTTOM CELL.
2.5 PROVIDE (1) #5 VERTICAL REINFORCING STEEL ELECTRICAL GROUND TO FOUNDATION STEEL.
2.6 FOUNDATION DOWELS AND VERTICAL REINFORCING SPACES AS SHOWN ON FLOOR PLANS. IN THE EVENT OF CONFLICTS, THE FLOOR PLANS SHALL TAKE PRECEDENCE OVER THE FOUNDATION PLAN.
2.7 ALL FOOTINGS TO BE SMOOTH AND LEVEL.
2.8 REINFORCING STEEL LAP LENGTH IN CONCRETE AND/OR MASONRY SHALL BE:
#5 REBAR –30"
#6 REBAR –36"
#7 REBAR –45"
2.9 LAP LENGTH OF INDIVIDUAL BARS WITHIN A BUNDLE SHALL BE THAT FOR THE INDIVIDUAL BAR, INCREASED 20% FOR THREE–BAR BUNDLE, AND 33% FOR FOUR–BAR BUNDLE.
2.10 INDIVIDUAL BARS WITHIN A BUNDLE TERMINATED WITHIN THE SPAN OF THE BEAM SHALL TERMINATE AT DIFFERENT POINTS WITH AT LEAST 40Db STAGGER.
2.11 A FILLED CELL WITH (1) #5 VERTICAL SHALL BE LOCATED AT GIRDER TRUSSES WITH UPLIFT EXCEEDING 2000LBS U.N.O.
2.12 MINIMUM CONCRETE COVER 3" CAST AGAINST SOIL AND 1½" ELSE U.N.O. MAXIMUM CONCRETE COVER 6" U.N.O.

4. WOOD NOTES

4.1 PSL: 1.8E PARALLEL STRAND LUMBER, Fb=2400psi
4.2 LVL: 1.9E LAMINATED VENEER LUMBER, Fb=2600psi
4.3 PT: PRESSURE TREATED SOUTHERN PINE #2 GRADE OR BETTER
4.4 SPF: SPRUCE PINE FIR #2 GRADE OR BETTER

6. DESIGN LOADS AND NOTES

6.1 NO PROVISION HAS BEEN MADE IN THE STRUCTURAL DESIGN FOR TEMPORARY CONDITIONS OCCURRING DURING CONSTRUCTION, UNLESS SPECIFICALLY NOTED ON THE STRUCTURAL DRAWINGS. THE CONTRACTOR SHALL PROVIDE ALL NECESSARY SHORING AND BRACING REQUIRED TO RESIST STRESSES OR INSTABILITY OCCURRING FROM ANY CAUSE DURING CONSTRUCTION. THE CONTRACTOR SHALL ASSUME COMPLETE RESPONSIBILITY FOR SUCH MEASURES.

7. WIND NOTES

7.1 WIND LOADS ARE BASED ON A WIND VELOCITY OF 140 MPH APPLIED FOR A FULLY ENCLOSED STRUCTURE.
7.2 –
7.3 WIND DESIGN LOADS WERE DETERMINED BASED ON THE FOLLOWING:
BASIC WIND SPEED = 140 MPH (LRFD),
BUILDING CATEGORY II,
WIND EXPOSURE = C,
INTERNAL PRESSURE COEFFICIENT = 0.18.
FULLY ENCLOSED BUILDING

DESIGN WIND PRESSURES FOR COMPONENTS AND CLADDING

POSITIVE PRESSURES = INWARD
NEGATIVE PRESSURES = OUTWARD (SUCTION)
ALL PRESSURE VALUES ARE IN PSF.

COMPONENT AREA (SQ. FT.)	ZONE 4	ZONE 5
10	+24.5/–26.6	+24.5/–32.7
20	+23.4/–25.5	+23.4/–30.5
30	+22.8/–24.9	+22.8/–29.2
40	+22.1/–24.3	+22.1/–28.4
50	+22.0/–24.0	+22.0/–27.6
75	+21.3/–23.4	+21.3/–26.3
100	+20.7/–22.8	+20.7/–25.5
150	+20.1/–22.2	+20.1/–24.0

8. STRUCTURAL STEEL NOTES

8.1 CHANNELS AND ANGLES: ASTM A36/A36M, ASTM A572/A572M, GRADE 50, HIGH STRENGTH, LOW–ALLOY, COLUMBIUM–VANADIUM STEEL.
8.2 PLATE AND BAR: ASTM A36/A36MASTM A572/A572M, GRADE 50, HIGH STRENGTH, LOW–ALLOY, COLUMBIUM–VANADIUM STEEL.
8.3 COLD–FORMED HOLLOW STRUCTURAL SECTIONS: ASTM A500, GRADE B STRUCTURAL TUBING.
8.4 STEEL PIPE: ASTM A501 OR A53, TYPE E OR S, GRADE B.
8.5 HIGH STRENGTH BOLTS, NUTS, AND WASHERS: ASTM A325, TYPE 1 HEAVY HEX STEEL STRUCTURAL BOLTS, ASTM A563, HEAVY HEX CARBON–STEEL NUTS, AND ASTM F436 HARDENED CARBON–STEEL WASHERS.
8.6 PRIMER: FABRICATOR'S STANDARD LEAD– AND CHROMATE–FREE, NONASPHALTIC, RUST–INHIBITING PRIMER.
8.7 GROUT: ASTM C1107 NONMETALLIC, SHRINKAGE RESISTANT, FACTORY PACKAGED.
8.8 FABRICATE AND ASSEMBLE STRUCTURAL STEEL IN SHOP TO GREATEST EXTENT POSSIBLE. FABRICATE ACCORDING TO AISC'S "CODE OF STANDARD PRACTICE FOR STEEL BUILDINGS AND BRIDGES" AND IT'S "SPECIFICATION FOR STRUCTURAL STEEL BUILDINGS – ALLOWABLE STRESS DESIGN AND PLASTIC DESIGN."
8.9 ERECTION: SET STRUCTURAL STEEL ACCURATELY IN LOCATIONS AND TO ELEVATIONS INDICATED ON PLANS IN ACCORDANCE WITH AISC'S "CODE OF STANDARD PRACTICE FOR STEEL BUILDING AND BRIDGES."
8.10 CONTRACTOR TO VERIFY ALL DIMENSIONS AND HEIGHTS PRIOR TO ERECTION.
8.11 WELDING: COMPLY WITH AWS D1.1/D1.1M FOR WELDING PROCEDURE SPECIFICATIONS, TOLERANCES, APPEARANCE, AND QUALITY.
8.12 WELDING CERTIFICATES TO BE ISSUED TO SPECIALTY ENGINEER FOR REVIEW.

10. GENERAL CONNECTIONS NOTES

10.1 CONNECTIONS SHOWN ARE RECOMMENDED, BUT OTHER CONNECTORS MAY BE SUBSTITUTED AS LONG AS THEY MEET OR EXCEED UPLIFTS AND LATERAL CAPACITY OF THE ANCHORS SPECIFIED AND SATISFY TRUSS LAYOUT REQUIREMENTS COMPLIANCE WITH USP, SIMPSON OR OTHER MANUFACTURER'S REQUIREMENTS.
10.2 FOR ADDITIONAL TIE DOWN INFORMATION, SEE SIMPSON OR USP CATALOGS.
10.3 FOR POST–INSTALLED ANCHORS: HOLE PREPARATION, CARTRIDGE PREPARATION, AND EPOXY FILLING SHALL BE PERFORMED PER MANUFACTURER'S ADHESIVE ANCHOR INSTALLATION INSTRUCTIONS.
10.4 AN EPOXY INSPECTION MAY BE REQUIRED DEPENDING ON JURISDICATION. CONTRACTOR MUST VERIFY.

13. ABBREVIATIONS

ABV. ABOVE
A.F.F. ABOVE FINISHED FLOOR
A.O.R. ANGLE OF REPOSE
C.M.U. CONCRETE MASONRY UNIT
E.O.R. ENGINEER OF RECORD
F&P FORMED & POURED
MANF. MANUFACTURER
MAX. MAXIMUM
MYE MCCALL & YOUNG ENGINEERING, LLC
O.C. ON CENTER
O.D. OUTER DIAMETER
OPG. OPENING
PLF POUNDS PER LINEAR FOOT
REINF. REINFORCING STEEL
REQ'S REQUIREMENTS
SIM. SIMILAR
STD. STANDARD
SS STAINLESS STEEL
STL. STEEL
T.O.C. TOP OF CONCRETE
T.O.P. TOP OF PLATE
U.N.O. UNLESS NOTED OTHERWISE
W/ WITH

14. REMODEL NOTES

14.1 ALL WOOD OR WOOD PRODUCTS IN CONTACT WITH CONCRETE OR MASONRY TO BE EITHER MOISTURE PROTECTED OR PRESSURE TREATED.
14.2 SHORING SHALL BE IN ACCORDANCE WITH OSHA REGULATIONS.
14.3 NOTIFY SPECIALTY ENGINEER IF ANY OF THE ASSUMED AS–BUILT CONDITIONS VARY.
14.4 ALL WATERPROOFING BY OTHERS.

15. WATERPROOFING NOTES

15.1 ALL WATERPROOFING, FLASHING, & MOISTURE PROTECTION IS THE RESPONSIBILITY OF THE GENERAL CONTRACTOR

17. COLD–FORMED METAL NOTES

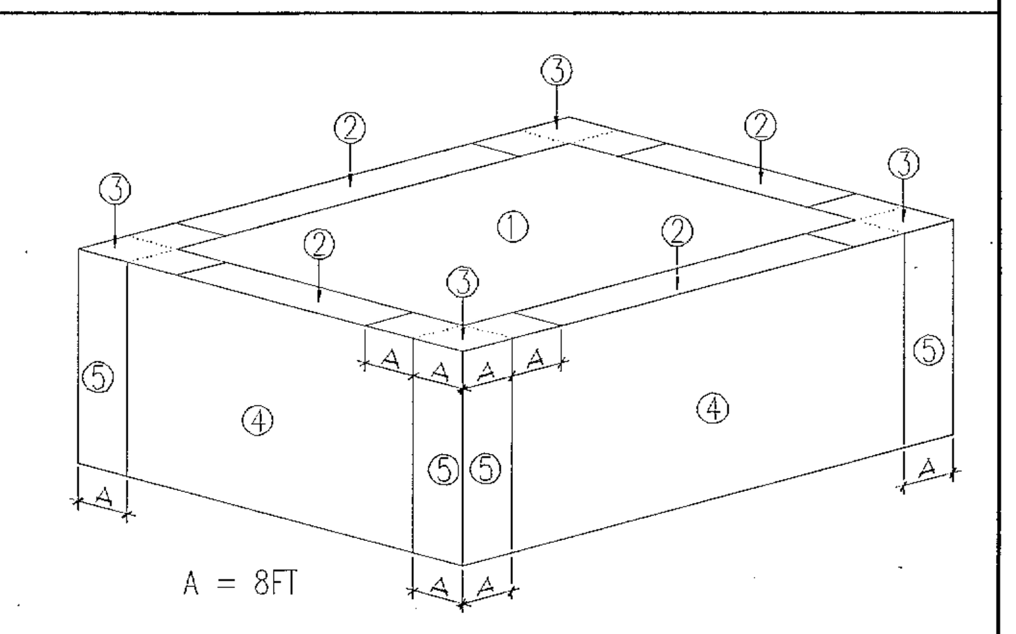
17.1 ALL FRAMING MEMBERS SHALL BE FORMED FROM CORROSION–RESISTANT STEEL, CORRESPONDING TO THE REQUIREMENTS OF ASTMA653, WITH A MINIMUM YIELD STRENGTH OF 33 KSI.
17.2 AXIALLY LOADED STUDS SHALL BE INSTALLED IN A MANNER THAT WILL ASSURE THAT THEIR ENDS ARE POSITIONED AGAINST THE INSIDE OF RUNNER WEB PRIOR TO FASTENING.
17.3 FASTENING OF COMPONENTS SHALL BE WITH SELF–DRILLING SCREWS. SCREWS SHALL BE OF SUFFICIENT SIZE TO ENSURE THE STRENGTH OF THE CONNECTION. WIRE TYING OF COMPONENTS SHALL NOT BE PERMITTED.
17.4 RUNNERS SHALL BE SECURELY ANCHORED TO THE SUPPORTING STRUCTURE.
17.5 TEMPORARY BRACING, WHERE REQUIRED, SHALL BE PROVIDED UNTIL ERECTION IS COMPLETED.
17.6 SPLICES IN STUDS SHALL NOT BE PERMITTED UNLESS DESIGNED BY A PROFESSIONAL ENGINEER AND SUBMITTED FOR REVIEW.

DESIGN WIND PRESSURES FOR COMPONENTS AND CLADDING

POSITIVE PRESSURES = INWARD
NEGATIVE PRESSURES = OUTWARD (SUCTION)
ALL PRESSURE VALUES ARE IN PSF.

COMPONENT AREA (SQ. FT.)	
	ZONE 1
10	+10.3/–25.4
	ZONE 2
	+10.3/–42.6
	ZONE 3
	+10.3/–64.1
SOFFIT PRESSURE	ZONE 4
	+24.5/–26.6
	ZONE 5
	+24.5/–32.7

ROOF AND WALL ZONES FOR COMPONENTS AND CLADDING WIND PRESSURES



TO THE BEST OF MY KNOWLEDGE, THE PLANS AND SPECIFICATIONS FOR THIS RESIDENCE COMPLY WITH THE APPLICABLE STRUCTURAL PROVISIONS OF THE 2017 EDITION OF THE FLORIDA BUILDING CODE

6.4.18
4.9.18
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A CUSTOM RENOVATION FOR:
ORLANDO GIFT SHOP
7403 INTERNATIONAL DRIVE
ORLANDO, FL

STRUCTURAL NOTES

REVISIONS
JLM 03/06/18

SHEET
S1